ERRATA SHEET
Roark's Formulas for Stress & Strain
6th edition, 1st printing

Note: For ready reference, existing text is shown on the left and corrected text on the right.

Table 1 Properties of Sections

Case 10 p.65	$t_1 = 2/3\ d$ $r_m = 0.2347 \sqrt{b^2 - ab + a^2}$	$y_1 = 2/3\ d$ $r_m = 0.2357 \sqrt{b^2 - ab + a^2}$
Case 11 p.65	$I = 1/12\ bd^3$	$I_i = 1/12\ bd^3$
Case 18 p.68	(*Note*: If $\alpha < \pi/4$ use expressions from case 19)	(*Note*: If $\alpha \leq \pi/4$ use expressions from case 19)
Case 22 p.69	$b = \dfrac{R + R_1}{2}$	$b = \dfrac{R + R_i}{2}$
Case 25 p.70	For I_2 interchange a and b in the expressions for I_2, K_2, and K_3	For I_2 interchange a and b in the expressions for I_1, K_2, and K_3
	For I_2 interchange a and b in the expressions for I_2, K_2, and K_3	[Delete these two duplicate lines]
Case 26 p.71	For I_2 use one– half the value for I_2 in case 17	For I_2 use one– half the value for I_2 in case 25
	For Z_2 use one-half the value for Z_2 in case 17	For Z_2 use one-half the value for Z_2 in case25
Case 27 p.72	For $n = 3$, see case 3. For $n = 4$, see cases 1 and 7.	For $n = 3$, see case 8. For $n = 4$, see cases 1 and 12.

Table 2 Formulas for combined stress

Case 6 p.89	plane normal to OX^1	plane normal to OX'

Table 3 Shear, moment, slope and deflection formulas for elastic straight beams

Case 4b p.108	$M_B = \dfrac{EI\theta_o}{l}$	$M_B = \dfrac{-EI\theta_o}{l}$

Table 7 Shear, moment, slope and deflection formulas for finite length beams on elastic foundations

Case 6, p.146 Free/Guided	$y_A = -\Delta_o \dfrac{C_1 C_{a2} + C_3 C_{A4}}{C_{12}}$	$y_A = -\Delta_o \dfrac{C_1 C_{a2} + C_3\ C_{a4}}{C_{12}}$

Table 7 Shear, moment, slope and deflection formulas for finite length beams on elastic foundations (continued)		
Case 7, p.147 Guided/Free	$y_A = \frac{-(T_1 - T_2)\gamma}{2\beta^2 t} \frac{C_4}{C_{12}}$	$y_A = \frac{(T_1 - T_2)\gamma}{2\beta^2 t} \frac{C_4}{C_{12}}$
Case 7, p.147 Simply Supported/ Free	$R_A = \ldots \frac{2C_1C_3 + C_4^2}{C_{13}}$	$R_A = \ldots \frac{C_4^2 + 2C_1C_3 - 2C_3}{C_{13}}$
Example 1 p.152	. . . $b_o k_o$ is 1.013/24 = 42.2 . . .	. . . $b_o k_o$ is 1,013/24 = 42.2 . . .
Example 3 p.154	$w_4 = -800$ lb/in	$w_4 = -80$ lb/in
Article 7.7 Beams under Simultaneous Axial and Transverse Loading		
Example 1 p.175		[Add $E = 30 \cdot 10^6$]
Example 2 p.180		[Units in Example 2 are cm^2 for A, cm^4 for I, and N/cm^2 for E]
Table 14 Position of flexural center Q for different sections		
Case 1 p.214		[Delete "For any equilateral triangle, e = 0"]
Case 5 p.214	$e = b^2h^2t/4I_x$	$e = h^2t(b^2 - t^2/4)/4I_x$
Table 15 Collapse loads with plastic hinge locations for straight beams		
Case 3a p.226	$M_c = M_p$	$M_{oc} = M_p$
Case 3b p.226	$M_c = 2M_p$	$M_{oc} = 2M_p$
Case 3f p.226		[Diagram is printed upside down]
Table 16 Formulas for curved beams subjected to bending in the plane of the curve		
Case 7 p.238	$d/c = 3/4\pi$ For $R_x > d$, $R/c > 3.356$ and For $R_x < d$, $R/c < 3.356$ and	$d/c = 3\pi/4$ For $R_x \geq d$: $R/c \geq 3.356$ and For $R_x < d$: $R/c < 3.356$ and

Table 16 Formulas for curved beams subjected to bending in the plane of the curve (continued)		
Case 7 p.238	$\ln \dfrac{d/c + (d/c)^2 - (R/c-1)^2}{R/c-1}$	$\ln \dfrac{d/c + \sqrt{(d/c)^2 - (R/c-1)^2}}{R/c-1}$ [expression is in last equation]
Case 9 p.240	For $R_x > a$, $R/c > (a/c)(1+\cos\alpha) + 1$ and For $R_x < a$, $R/c < (a/c)(1+\cos\alpha) + 1$ and	For $R_x \geq a$: $R/c \geq (a/c)(1+\cos\alpha) + 1$ and For $R_x < a$: $R/c < (a/c)(1+\cos\alpha) + 1$ and
Case 11 p.241	c_1/c_2	c_1/c [expression is in first equation]
Article 8.1 Bending in the Plane of the Curve		
Ex. 2 p. 248	. . . and from case 18 of Table 1, . . .	. . . and from case 19 of Table 1, . . .
Table 17 Formulas for circular rings		
7th line p.262	. . . zero to π for all cases except 16 and 17, . . .	. . . zero to π for all cases except 18 and 19, . . .
Case 2 p.263		[Add closing square bracket to equation for D_V]
Case 3 p.264	$\Delta L = \dfrac{M_0 R^2}{EI} \ldots$ $\Delta L_w = -\dfrac{-M_0 R^2}{EI\pi} \ldots$	$\Delta L = \dfrac{-M_0 R^2}{EI} \ldots$ [1st equation for ΔL] [Add closing square bracket to 2nd equation for ΔL] $\Delta L_w = \dfrac{-M_0 R^2}{EI\pi} \ldots$
Case 5 p.266	$\Delta L = \dfrac{WR^3}{EI2} \ldots$	$\Delta L = \dfrac{WR^3}{2EI} \ldots$ [both equations for ΔL_w]
Case 7 p.268	Max $-M = -WR(k_2/\theta - c/s)$	Max $-M = -(1/2)WR(k_2/\theta - c/s)$
Case 9 p.269	$M_C = \ldots 3s(s - \pi - \theta) \ldots$	$M_C = \ldots 3s(s - \pi + \theta) \ldots$
Case 11 p.271	$\Delta L = \begin{cases} \ldots \text{[1st equation]} \ldots & \text{for } \theta \leq \pi/2 \\ \ldots \text{[2nd equation]} \ldots & \text{for } \theta \leq \pi/2 \end{cases}$	$\Delta L = \begin{cases} \ldots \text{[1st equation]} \ldots & \text{for } \theta \leq \pi/2 \\ \ldots \text{[2nd equation]} \ldots & \text{for } \theta \geq \pi/2 \end{cases}$
Case 14 p.274	$M_C = \ldots \pi(\pi + \theta)^2 \ldots$ $D_H = \ldots k_1(2 - c) \ldots$	$M_C = \cdots \pi(\pi - \theta)^2 \cdots$ $D_H = \ldots k_1[(2 - c) \ldots$ [2nd equation for D_H]

Table 17 Formulas for circular rings (continued)		
Case 15 p.275	where $K_T = 1 - I/(AR^2)$ $LT_M = -wR^2(xz + u - 1)$	where $K_T = 1 + I/(AR^2)$ $LT_M = -wR^2[xz + K_T(u - 1)]$
Case 21 p.281	*Note:* The constant K_t accounts for . . .	*Note:* The constant K_T accounts for . . . [Refer to lines 3–4 and 15–16 on p. 262 for the distinction between the variable *A* used in the equations and the point A shown on the diagram.]
Table 18 Reaction and deformation formulas for circular arches		
[Cases 1-4] p.286		[Add the equation $A_{MH} = A_{HM}$ to the discussion of general reaction and expressions for cases 1 to 4]
Case 1h p.289	−0.2684	−0.2648 [2nd row, 2nd column] ["uniformly" is printed as "uniformity"]
Case 1i p.289	$LP_M = \ldots k_2(2\theta^2 s - \theta - sc) \ldots$	$LP_M = \ldots k_2(2\theta s^2 - \theta - sc) \ldots$
[Cases 5-14] p.293	Vertical deflection at $A = \delta_A = \ldots$	Vertical deflection at $A = \delta_{VA} = \ldots$
Case 5h p.296	For $\alpha = \beta = 0$	For $\alpha = \beta = 0$ and $R_{cg} = R$
Case 5i p.297	For $\alpha = \beta = 0$	For $\alpha = \beta = 0$ and $R_{cg} = R$
Table 19 Formulas for curved beams of compact cross section loaded normal to the plane of curvature		
Case 2j p.320	$V_A = -\dfrac{M_o \cos(\phi - \theta}{R \sin\phi}$	$V_A = -\dfrac{M_o \cos(\phi - \theta)}{R \sin\phi}$
Case 3g p.324	$M_B = \ldots - T_a \sin\phi \ldots$	$M_B = \ldots - T_A \sin\phi \ldots$
Article 8.5 Curved Beams Loaded Normal to Plane of Curvature		
Example 2 p.339	...a load of 3000 lb is 40° out from the wall.	...a load of 3000 lb is 20° out from the wall.
Example 2 p.340	$\dfrac{VQ}{Ib} = \dfrac{(1000 - 359.3)\ldots}{\ldots} = 487\ \text{lb/in}^2$	$\dfrac{VA'z'}{Ib} = \dfrac{(3000 - 359.3)\ldots}{\ldots} = 2008\ \text{lb/in}^2$

Table 20 Formulas for torsional deformation and stress		
Case 11 p.351	$K = \pi(D^4 - d^4)/32Q$ $Q = \ldots$	$K = \pi(D^4 - d^4)/32C$ $C = \ldots$
Cases 19-26 p.354	. . . and D, A, and r have the same meaning as before and ϕ = angle through which . . .	. . . and D, A, and r have the same meaning as before and ϕ = a positive angle through which . . .
Case 28 p.356	$B - K_1 + K_2\, b/r \ldots$	$B = K_1 + K_2\, b/r \ldots$
Case 29 p.356	$K_4 = \ldots - 1.1053\, a_2/b$	$K_4 = \ldots - 1.1053\, (a/b)^2$
Table 22 Formulas for the elastic deformations of uniform thin—walled open members under torsional loading		
Case 1c p.372	$T_B - -T_o$	[Delete the equation $\theta_A''' = 0$] $T_B = -T_o$
Case 2c p.376		[Delete the equation $\theta_A''' = 0$]
Article 9.3 Effect of End Constraint		
Example 1 p.379	Assume $E = 30(10^3)\ \text{lb/in}^2 \ldots$	Assume $E = 30(10^6)\ \text{lb/in}^2 \ldots$
Table 24 Formulas for flat circular plates of constant thickness		
Case 2g p. 407	0.1226 0.0221 0.0048 0.0007	−0.1226 −0.0221 −0.0048 −0.0007
Case 4g p.415	0.0285 0.0049 0.0010 0.00015	−0.0285 −0.0049 −0.0010 −0.00015
Case 5 p.417		[Add the following after the equation for M_r] *Note:* If the loading is on the inside edge, $r > r_o$ everywhere and $\langle r - r_o \rangle^0 = 1$
Case 5c p.418	11.4835 4.3830 3.6964 4.5358 10.9401	−11.4835 −4.3830 −3.6964 −4.5358 −10.9401
Case 6 p.420	For the numerical data given below, $\nu = 0.3$	For the numerical data given below, $\nu = 0.3$, and all values given for $K_{M_{to}}$ are found just outside r_o

Table 24 Formulas for flat circular plates of constant thickness (continued)

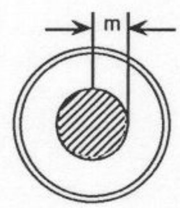
Case 6b p. 420	0.3133 0.1522 0.1349 0.0391 0.0351	−0.3133 −0.1522 −0.1349 −0.0391 −0.0351
Case 7g p. 424	3168.165	3186.165 [Last row, 5th column]
Case 8 p.425		[Add following to general discussion] *Note:* If the temperature difference ΔT occurs over the entire plate, $r > r_o$ everywhere, so $<r - r_o>^0 = 1$ everywhere; therefore, all numerical data for $K_{M_{tb}}$ are found just to the outer side of b.
Case 8f p.427	If $r_o = b$ (ΔT over entire plate), (All deflections . . . in the plate)	If $r_o = b$ (ΔT over entire plate), all deflections . . . in the plate. If $r_o > b$, the following tabulated values apply.
Case 8h p.428	If $r_o = b$ (ΔT over entire plate), (All deflections . . . in the plate)	If $r_o = b$ (ΔT over entire plate), all deflections . . . in the plate. If $r_o > b$, the following tabulated values apply.
Case 12a p.430	$Q_a = \ldots - 12ar_o \ldots$	$Q_a = \ldots - 2ar_o \ldots$
Case 14b p.431	... ... -0.10078 -0.28396 -0.50000	... ... 0.10078 0.28396 0.50000
Case 15a p.432		[The column for $r_o/a = 0.0$ should have no value in the row for $K_{M_{to}}$. Also, add the following:] Note: Values for $K_{M_{to}}$ are found just to the outer side of r_o. When the entire plate is subjected to the temperature differential, there is no stress anywhere in the plate.

Table 24 Formulas for flat circular plates of constant thickness (continued)

Case 15b p.432		[The column for $r_o/a = 0.0$ should have no value in the rows for $K_{M_{ro}}$ and $K_{M_{to}}$. Also, add the following:] Note: Values for $K_{M_{to}}$ are found just to the outer side of r_o. When the entire plate is subjected to the temperature differential, the moments are the same everywhere in the plate and there are no deflections.
Case 16 p.432	For $r > r_o$ or $r_o' = r_o$ if $r_o > 0.5t$	For $r > r_o'$ or $r_o' = r_o$ if $r_o \geq 0.5t$
Case 17 p.433	For $r > r_o$ or $r_o' = r_o$ if $r_o > 0.5t$	For $r > r_o'$ or $r_o' = r_o$ if $r_o \geq 0.5t$
Case 18 p.433	or $r_o' = r_o$ if $r_o > 0.5t$	or $r_o' = r_o$ if $r_o \geq 0.5t$
Case 19 p. 434	$M_r = \ldots = \max M$ if $r_o < 0.6(a - p)$ $M_r = \ldots = \max M$ if $r_o > 0.6(a - p)$	$M_r = \ldots = \max M$ if $r_o' < 0.6(a - p)$ $M_r = \ldots = \max M$ if $r_o' > 0.6(a - p)$
Case 26 p.437	$\text{Max } y = \alpha_1 \dfrac{qa^4}{Et^3} \ldots$ $\text{Max } y = \alpha_2 \dfrac{qa^4}{Et^3} \ldots$	$\text{Max } y = -\alpha_1 \dfrac{qa^4}{Et^3} \ldots$ $\text{Max } y = -\alpha_2 \dfrac{qa^4}{Et^3} \ldots$

Table 26 Formulas for flat plates with straight boundaries and constant thickness

Fifth line p.458	$\ldots$ if $r_o > 0.5t \ldots$	$\ldots$ if $r_o \geq 0.5t \ldots$
Case 11d p.472	$(\ldots$ if $b/2 < a < b \ \ldots)$	$(\ldots$ if $b/2 \leq a < b \ \ldots)$
Case 12a p.473	$\ldots$ when $0.15 < n < 0.30$	$\ldots$ when $0.15 \leq n < 0.30$

Article 10.11		
p. 480	3.7? [column for $qb^4/Et^4 = 50$, middle row for $a/b = 1.5$]	3.78
	13.20 [column for $qb^4/Et^4 = 250$, middle row for $a/b = 2$ to ∞]	12.20
Table 27	**Formulas for short prisms loaded eccentrically; stress reversal impossible**	
Case 2 p.506	If $\frac{a+d}{2} < x < d$	If $\frac{a+d}{2} \le x < d$ [In both occurrences]
Case 4 p.507	$M = \frac{r}{4}$	$m = \frac{r}{4}$
Case 6 p.508		[Add the definition for m to the diagram] 
Table 28	**Formulas for membrane stresses and deformations in thin—walled pressure vessels**	
Case 2b p.521	At any level y below the liquid surface, $y < d$ $\sigma_2 = \frac{t(d-y)\delta \tan \alpha}{t \cos \alpha}$	At any level y below the liquid surface, $y \le d$ $\sigma_2 = \frac{y(d-y)\delta \tan \alpha}{t \cos \alpha}$
Article12.2	**Thin Shells of Revolution under Distributed Loadings Producing Membrane Stresses Only**	
Ex. 2 p.528	$\Delta h = \frac{17{,}360 \ln (20/11.96)}{2\pi(10)(10^6) \sin 15° \cos^2 15°} = 0.000588$ in	$\Delta h = \frac{17{,}360 \ln (20/11.96)}{2\pi(10)(10^6)(0.25) \sin 15° \cos^2 15°} = 0.002353$ in
	$\Delta h = -0.00628 + 0.00225 + 0.000588 = -0.0344$ in	$\Delta h = -0.00628 + 0.00225 + 0.002345 = -0.00168$ in

Article 12.3 Thin Shells of Revolution under Concentrated or Discontinuous Loadings Producing Bending and Membrane Stresses		
Ex. 1 p.529	$\sigma^2 = \frac{yE}{Ra} + \upsilon\sigma_1$	$\sigma_2 = \frac{yE}{R} + \upsilon\sigma_1$
Table 29 Shear, moment, slope, and deflection formulas for long and short thin—walled cylindrical shells under axisymmetric loading		
Case 13 p.534	$LT_y = \frac{-q}{4D}\left[\ldots + \frac{F_{b1}}{\lambda^4}\right]$	$LT_y = \frac{-q}{4D}\left[\ldots - \frac{F_{b5}}{\lambda^4}\right]$
Cases 15–17 p.535		[The discussion on p.535 applicable to Cases 15–17 should include the condition that $R > 10t$]
Article 12.3 Thin Shells of Revolution under Concentrated or Discontinuous Loadings Producing Bending and Membrane Stresses		
p.543 Line 19	In Table 30, cases 4 and 5 for long cones, where the...	In Table 30, case 4 for long cones, where the...
Table 30 Formulas for bending and membrane stresses and deformations in thin—walled pressure vessels		
Case 1c p.545	$\sigma'_2 = \frac{2\Delta_0 E\upsilon}{t\beta^2 K_2 \sin\phi}$	$\sigma'_2 = \frac{3\Delta_0 E\upsilon}{t\beta^2 K_2 \sin\phi}$
Case 2 p.546	Note: ... $r'_0 < 0.5t$; ... if $r_0 > 0.5t$	Note: ... $r_0 < 0.5t$; ... if $r_0 \geq 0.5t$
Case 2 p.546 Table	μ | 0.1 0.2 0.4 0.6 ...	μ | 0 0.1 0.2 0.4 0.6 ...
Case 3 p.547	...coefficients the depend upon...	...coefficients that depend upon...
Case 5c p.560	$\Delta R = \frac{\ldots \sin^2\alpha}{\ldots} \ldots$	$\Delta R = \frac{\ldots \sin\alpha}{\ldots} \ldots$

Table 30	**Formulas for bending and membrane stresses and deformations in thin–walled pressure vessels (continued)**	
Case 5d p.566	$K_{\Delta h2} = 27.998$	$K_{\Delta h2} = -27.998$
Case 8b p.573	Max $\sigma_1 = -0.156 \ldots$	Max $\sigma_1 = -0.153 \ldots$
Article12.3	**Thin Shells of Revolution under Concentrated or Discontinuous Loadings Producing Bending and Membrane Stresses**	
p.574 Line 6	. . . cases 4 and 5 may be used.	. . . case 4 may be used.
Table 31	**Formulas for discontinuity stresses and deformations at the junctions of shells and plates**	
Case 1 p.584	$C_{AB1} = \dfrac{-E_1 t_1}{sD_1 \lambda_1^2}$	$C_{AB1} = \dfrac{-E_1 t_1}{2D_1 \lambda_1^2}$
Case 2 p.589	$b_2 = R_1 - (t_2 \cos \alpha_2)/2$ $a_2 = R_1 + (t_2 \cos \alpha_2)/2$	$b_2 = R_2 - (t_2 \cos \alpha_2)/2$ $a_2 = R_2 + (t_2 \cos \alpha_2)/2$ Where R_2 is the mid–thickness radius of the cone at the junction.
Case 2b p.591	$LT_{A2} = \ldots$ $LT_{B2} = \dfrac{E_1 R_1 \tan \alpha_2}{2E_2 t_2 \cos \alpha_2}$	$LT_{A2} = \ldots + \dfrac{R_1 C_{AA2}}{2t_1} \tan \alpha_2$ $LT_{B2} = \dfrac{E_1 R_1^2 \tan \alpha_2}{2E_2 R_2 t_2 \cos \alpha_2} + \dfrac{R_1 C_{AB2}}{2t_1} \tan \alpha_2$ [The tabulated values are invalid unless the book already has these corrections for LT_{A2} and LT_{B2}]
Case 2c p.592	$KT_{B1} = \dfrac{-b_1 R_1}{x_1 t_1}$	$LT_{B1} = \dfrac{-b_1 R_1}{x_1 t_1}$
Case 3 p.594	See Table 30 for formulas for D_1 and λ_1.	See Table 29 for formulas for D_1 and λ_1.

Table 31 Formulas for discontinuity stresses and deformations at the junctions of shells and plates (continued)

Case 3b p.596	$LT_{A2} = \ldots$	$LT_{A2} = \ldots + \dfrac{R_1 C_{AA2}}{2t_1 \tan \phi_2}$
	$LT_{B2} = 0$	$LT_{B2} = \dfrac{R_1 C_{AB2}}{2t_1 \tan \phi_2}$ [The tabulated values are invalid unless the book already has these corrections for LT_{A2} and LT_{B2}]
Case 3c p.597	$LT_{B1} = \dfrac{-bR_1}{x_1 t_1}$	$LT_{B1} = \dfrac{-b_1 R_1}{x_1 t_1}$
Case 4b p. 601	$\ldots - \ln \dfrac{R_2}{R_1}$	$\ldots - 2 \ln \dfrac{R_2}{R_1}$ [The tabulated values are not affected by this correction to the equation defining K_{P2} for $R_2 \geq R_1$]
Case 4c p.602	$LT_{AC} = \dfrac{E_1 t_2 b_1^2}{32 D_2 t_1 R_1} K_{P2}$	$LT_{A2} = \dfrac{E_1 t_2 b_1^2}{32 D_2 t_1 R_1} K_{P2}$
Case 4d p.603 Table	4.4872	4.8472 [Last Column, 5th Row]
Case 5 p.604	$a_1 = R_1 + (t \cos \alpha_1)/2$ or $a_1 = R_1 = (t \cos \alpha_1)/2$	$a_1 = R_1 + (t_1 \cos \alpha_1)/2$
Case 5b p.606	$LT_{A2} = \ldots$	$LT_{A2} = \ldots + \dfrac{R_1 C_{AA2}}{2t_1} (\tan \alpha_1 + \tan \alpha_2)$
	$LT_{B2} = \ldots$	$LT_{B2} = \ldots + \dfrac{R_1 C_{AB2}}{2t_1} (\tan \alpha_1 + \tan \alpha_2)$ [The tabulated values are invalid unless the book already has these corrections for LT_{A2} and LT_{B2}]

Table 31 Formulas for discontinuity stresses and deformations at the junctions of shells and plates (continued)

Case 5c p.607	$LT_{A2} = \frac{-bR_2E_1}{E_2t_1t_2\cos\alpha_2}$	$LT_{A2} = \frac{-b_2R_2E_1}{E_2t_1t_2\cos\alpha_2}$
Case 6b p.611	$LT_{A2} = \ldots$ $LT_{B2} = 0$	$LT_{A2} = \ldots + \frac{R_1C_{AA2}}{2t_1}\left(\tan\alpha_1 + \frac{1}{\tan\phi_2}\right)$ $LT_{B2} = \frac{R_1C_{AB2}}{2t_1}\left(\tan\alpha_1 + \frac{1}{\tan\phi_2}\right)$ [The tabulated values are invalid unless the book already has these corrections for LT_{A2} and LT_{B2}]
Case 6c p.612	$LT_{B1} = \frac{-b_1}{t_1\cos\alpha l}\left(\frac{R_1}{x_1} + 2\tan\alpha_1\right)$	$LT_{B1} = \frac{-b_1}{t_1\cos\alpha_1}\left(\frac{R_1}{x_1} + 2\tan\alpha_1\right)$
Case 6d p.613	$\psi_A = \frac{\delta_1\omega_2R_1^2}{E_1}K_{\psi A}$	$\psi_A = \frac{\delta_1\omega^2R_1^2}{E_1}K_{\psi A}$
Case 7a p.615		[Under "Selected values", add:] Note: No correction terms are used.
Case 7b p.616	$LT_{A2} = \ldots$ $\ldots - \ln\frac{R_2}{R_1}$ $LT_{B2} = \ldots$ $N_1 = \frac{P}{2\pi R_1} - V_1\sin\alpha_1$ $\psi_A = \frac{P}{E_1\pi R_1^2}(K_{V1}C_{AB1} + K_{M1}C_{BB1})$	$LT_{A2} = \ldots + \frac{R_1C_{AA2}}{2t_1}\tan\alpha_1$ $\ldots - 2\ln\frac{R_2}{R_1}$ [in equation defining K_{p2} for $R_2 \geq R_1$] $LT_{B2} = \ldots + \frac{R_1C_{AB2}}{2t_1}\tan\alpha_1$ [The tabulated values are invalid unless the book already has these corrections for LT_{A2} and LT_{B2}] $N_1 = \frac{P}{2\pi R_1\cos\alpha_1} - V_1\sin\alpha_1$ $\psi_A = \frac{P}{E_1\pi R_1^2}(-LT_{B1} + K_{V1}C_{AB1} + K_{M1}C_{BB1})$

Table 31 Formulas for discontinuity stresses and deformations at the junctions of shells and plates (continued)

Case 7c p.617		[Under "Selected values", add:] Note: No correction terms are used.
Case 8b p.621	$LT_{A2} = \ldots$	$LT_{A2} = \ldots + \dfrac{R_1 C_{AA2}}{2t_1 \sin\phi_1}\left(\dfrac{1}{\tan\phi_1} + \dfrac{1}{\tan\phi_2}\right)$
	$LT_{B2} = 0$	$LT_{B2} = \dfrac{R_1 C_{AB2}}{2t_1 \sin\phi_1}\left(\dfrac{1}{\tan\phi_1} + \dfrac{1}{\tan\phi_2}\right)$ [The tabulated values are invalid unless the book already has these corrections for LT_{A2} and LT_{B2}]
	$V_1 = \dfrac{PtK_{V1}}{\pi R_1^2}$	$V_1 = \dfrac{Pt_1 K_{V1}}{\pi R_1^2}$
	$M_1 = \dfrac{Pt^2 K_{M1}}{\pi R_1^2}$	$M_1 = \dfrac{Pt_1^2 K_{M1}}{\pi R_1^2}$
	$N_1 = \dfrac{P}{2\pi R_2 \sin^2\phi_1} - V_1\cos\phi_1$	$N_1 = \dfrac{P}{2\pi R_1 \sin^2\phi_1} - V_1\cos\phi_1$
Case 9 p.624	$\sigma'_2 = \dfrac{V_1\beta_1^2\cos\phi_1}{K_1R_1} - \dfrac{6M_1}{t_1^2K_1}\left(\upsilon_1 + \dfrac{1-\upsilon_1/2}{\beta_1\tan\phi_1}\right)$	$\sigma'_2 = \dfrac{V_1\beta^2\cos\phi_1}{K_1R_1} - \dfrac{6M_1}{t_1^2K_1}\left(\upsilon_1 + \dfrac{1-\upsilon_1/2}{\beta\tan\phi_1}\right)$
Case 9a p.625		[Under "Selected values", add:] Note: No correction terms are used.
Case 9b p.626	$LT_{A2} = \dfrac{E_1t_2R_A^3}{16D_2t_1}K_{P2}$	$LT_{A2} = \dfrac{E_1t_2R_AR_1^2}{16D_2t_1}K_{P2} + \dfrac{R_1C_{AA2}}{2t_1\sin\phi_1}\ \dfrac{1}{\tan\phi_1}$

Table 31 Formulas for discontinuity stresses and deformations at the junctions of shells and plates (continued)

Case 9b p.626	$\ldots - \ln \frac{R_2}{R_1}$	$\ldots - 2 \ln \frac{R_2}{R_1}$ [in equation defining K_{P2} for $R_2 \geq R_1$]
	$LT_{B2} = \frac{E_1 R_A^3}{8D_2} K_{P2}$	$LT_{B2} = \frac{E_1 R_A R_1^2}{8D_2} K_{P2} + \frac{R_1 C_{AB2}}{2t_1 \sin \phi_1} \frac{1}{\tan \phi_1}$ [The tabulated values are invalid unless the book already has these corrections for LT_{A2} and LT_{B2}]
Case 9c p.627	. . . use the expressions from case 7a.	. . . use the expressions from case 9a. [All occurrences] [Under "Selected values", add:] Note: No correction terms are used.
Case 9d p.628 Table	$\frac{t_2}{t_1}$ | 15 30 50 | 15 30 15	$\frac{t_2}{t_1}$ | 15 30 50 | 15 30 50
Ex.3 p.633	$\Delta R_A = 546.75(10^{-6})$ $\psi_A = -630.38(10^{-6})$ $\sigma_1 = -59.264$ $\sigma_2 = 91.572$ $\sigma_1' = 1320.6$ $\sigma_2' = 285.21$	$\Delta R_A = 496.25(10^{-6})$ $\psi_A = -606.03(10^{-6})$ $\sigma_1 = -57.973$ $\sigma_2 = 81.858$ $\sigma_1' = 1258.4$ $\sigma_2' = 272.04$
p.634	$\Delta R_A = 626.88(10^{-6})$ $\psi_A = -699.19(10^{-6})$ $\sigma_1 = -56.236$ $\sigma_2 = 108.51$ $\sigma_1' = 1290.0$ $\sigma_2' = 282.7$	$\Delta R_A = 576.38(10^{-6})$ $\psi_A = -674.84(10^{-6})$ $\sigma_1 = -54.945$ $\sigma_2 = 98.792$ $\sigma_1' = 1227.8$ $\sigma_2' = 269.5$
	0.00063 in 0.000699 rad -391.2 lb/in^2 -1234 lb/in^2	0.00058 in 0.000675 rad 368.3 lb/in^2 1173 lb/in^2

Table 32 Formulas for thick—walled vessels under internal and external loading

Case 1b p.638	(σ_2, σ_3, and the max shear stress are the same as for case 1a)	(σ_2, σ_3, max σ_2, max σ_3, and the max shear stress are the same as for case 1a)
Case 1c p.638	Max shear stress $= \frac{\max \sigma_2}{2} = \frac{qa^2}{a^2 - b^2}$	Max shear stress $= \frac{\max \sigma_2}{2} = \frac{-qa^2}{a^2 - b^2}$
Case 1d p.639	(σ_2, σ_3, and the max shear stress are the same as for case 1c)	(σ_2, σ_3, max σ_2, max σ_3, and the max shear stress are the same as for case 1c)

Article 12.6 Thick Shells of Revolution		
Ex. 1 p.641	$\sigma_3 = \ldots \dfrac{3.85^2 + 2.425^2}{3.85^2 - 1.605^2} = -10{,}200$	$\sigma_3 = \ldots \dfrac{3.85^2 - 2.425^2}{3.85^2 - 1.605^2} = -10{,}200$
Table 35 Formulas for elastic stability of plates and shells		
Notation p.684		[Add the following:] For the plates, the smaller width should be greater than 10 times the thickness unless otherwise specified.
Case 1 p.684	. . . or unit external pressure.	. . . or external pressure
Case 3a p.685	$\alpha = \dfrac{\sigma_v}{\sigma_0 - \sigma_v}$ or $\alpha = \dfrac{\sigma_0}{\sigma_0 - \sigma_x}$	$\alpha = \dfrac{\sigma_0}{\sigma_0 - \sigma_v}$
Table	4.25	4.23 [3rd Column, 6th Row]
Case 7 p.687	$P = \dfrac{\pi}{3} \dfrac{Et^3}{(1-\upsilon^2)b}$ $P = \dfrac{2\pi}{3} \dfrac{Et^3}{(1-\upsilon^2)b}$	$P' = \dfrac{\pi}{3} \dfrac{Et^3}{(1-\upsilon^2)b}$ $P' = \dfrac{2\pi}{3} \dfrac{Et^3}{(1-\upsilon^2)b}$
Case 23 p.691	. . .to the meridian $(R_A + R_B)/2 \cos \alpha$ is . . .	. . . to the meridian $(R_A + R_B)/(2 \cos \alpha)$ is . . .
Case 25 p.692	$k_B < 150$	$k_B \leq 150$
Table 36 Natural frequencies of vibration for continuous members		
Case 13 p.717	$\beta a/D$	$\beta r/D$ [In both occurrences]

Table 37 Factors of stress concentration for elastic stress (k)

Case 7c p.733		[See Case 6b for interpretation of M_2.]
Case 10b p.735	. . .and $0.4 \le 2a/D \le 1.0$	. . .and $0.4 \le 2a/D < 1.0$
Case 13 p.736	. . .see cases 9b, 10b, and 11b.	. . .see case 10b.
Case 23 p.741	Ply/I	PLy/I